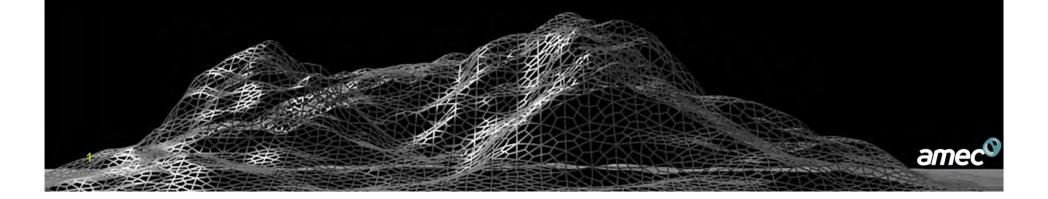
# 11<sup>th</sup> Annual Technical Forum GEOHAZARDS IMPACTING TRANSPORTATION IN THE APPALACHIAN REGION

### GEO-DESIGN APPLICATIONS IN KARST ENVIRONMENTS

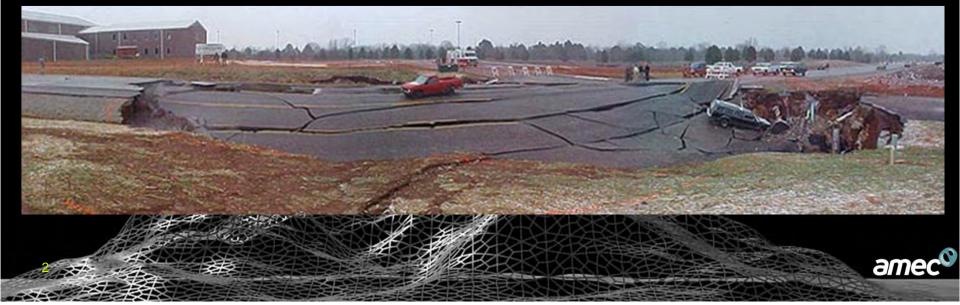
William D. Spencer, P.G., Jaye Richardson, and Chris Ramsey, P.E. AMEC, Nashville TN August 3, 2011



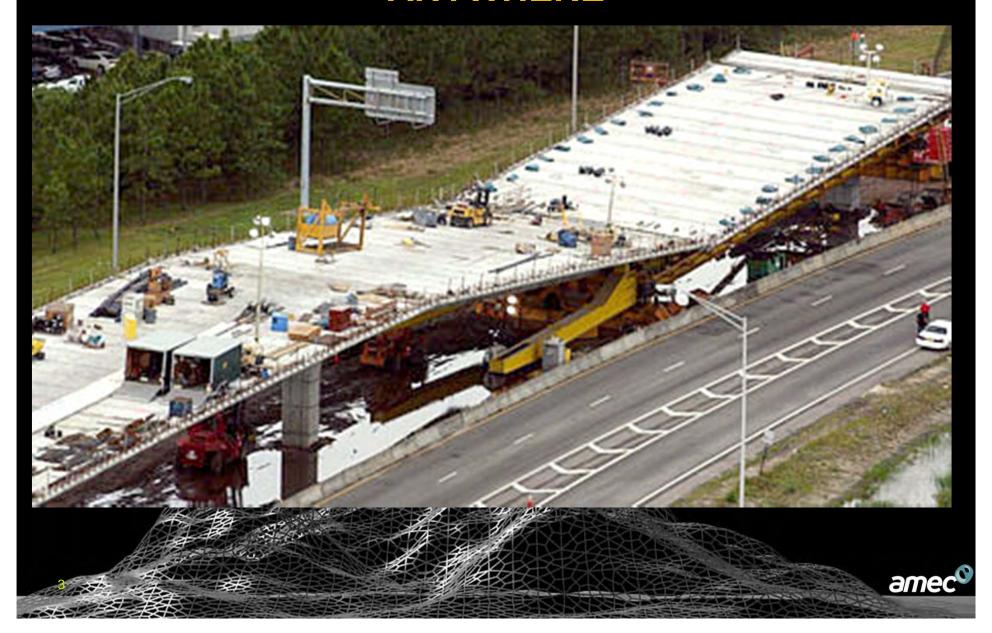
# KARST CAN OCCUR ANYTIME AND ANYWHERE

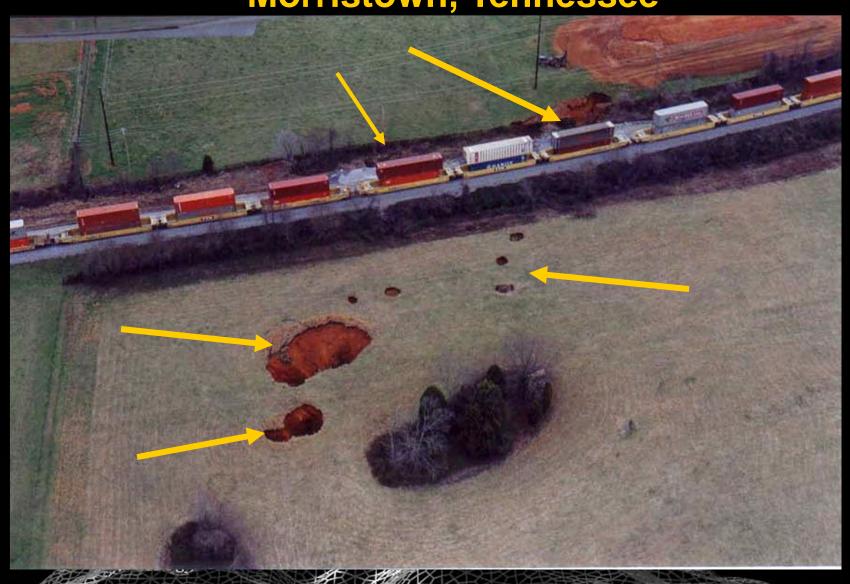


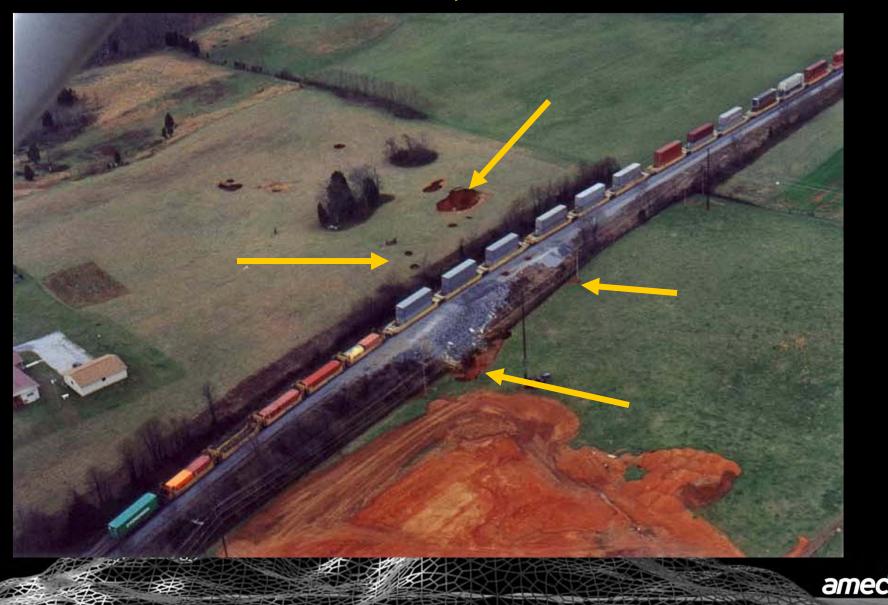


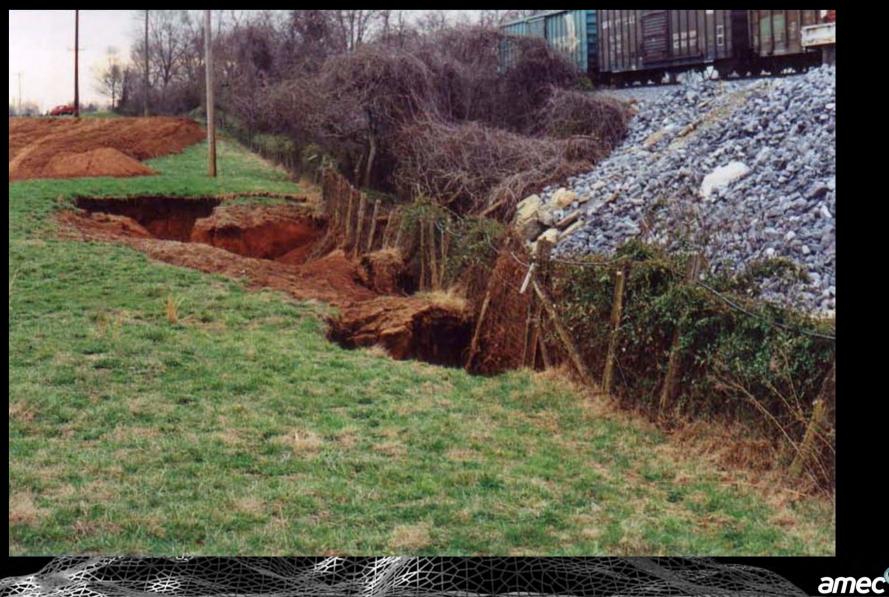


# KARST CAN OCCUR ANYTIME AND ANYWHERE













### **SUMMARY OF DEEP FOUNDATION OPTIONS**

	OPTIONS	PERCEIVED ADVANTAGES	PERCEIVED DISADVANTAGES
•	Driven Piles (H-Piles)	Low cost	Impossible to assess bearing strata
•	Drilled Piers (Caissons)	High load capacity; low long term risk	High cost; potentially large voids in bedrock might take large quantities of concrete to fill.
D	Small Diameter Prilled Pipe s (Micro Piles)	Low long-term risk; easy to make adjustments	Moderate to high cost; smaller than "normal" pile size for bridge bents; requires real-time field engineering during construction to adjust.
	4		



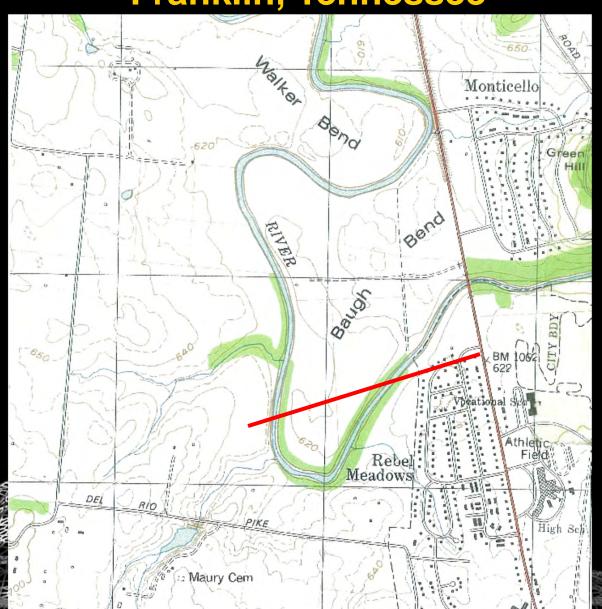




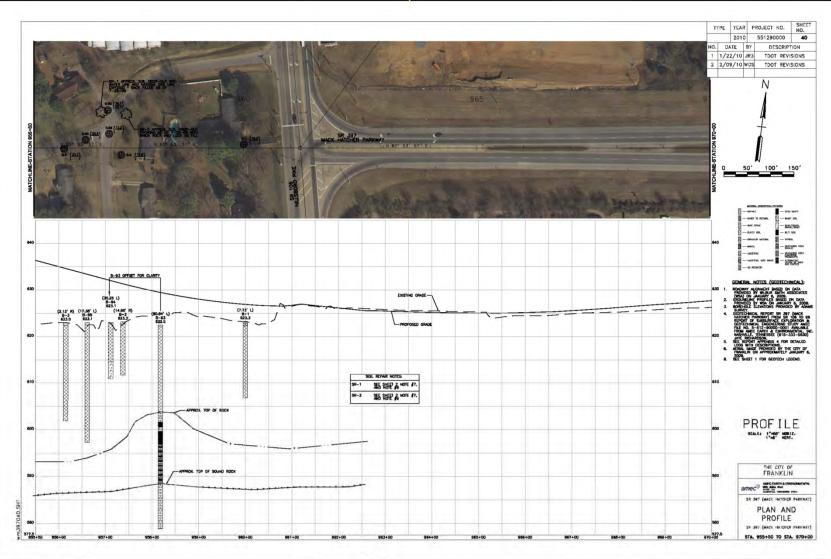
BEFORE
Karst engulfing the soil and ballast

AFTER
Micro pile and structural land bridge
REMOVES the karst from play and
provides for surface drainage
under the track.

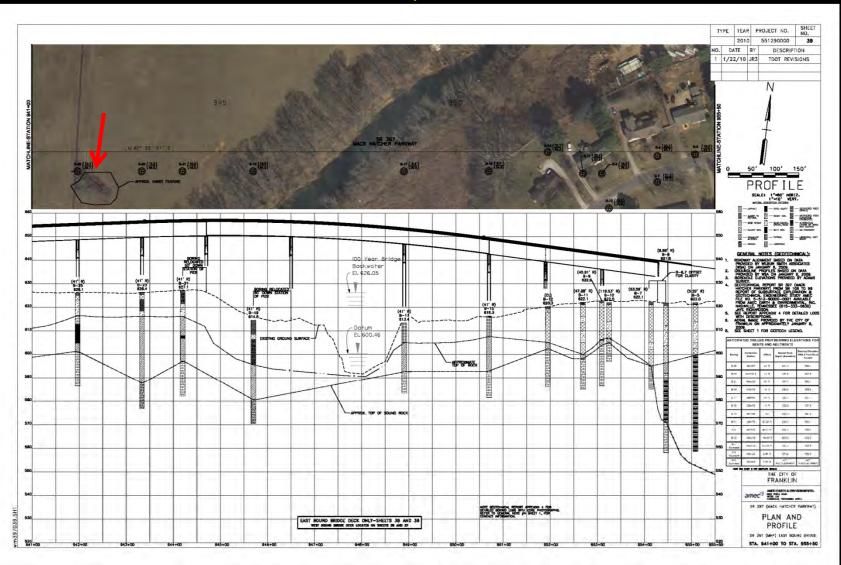




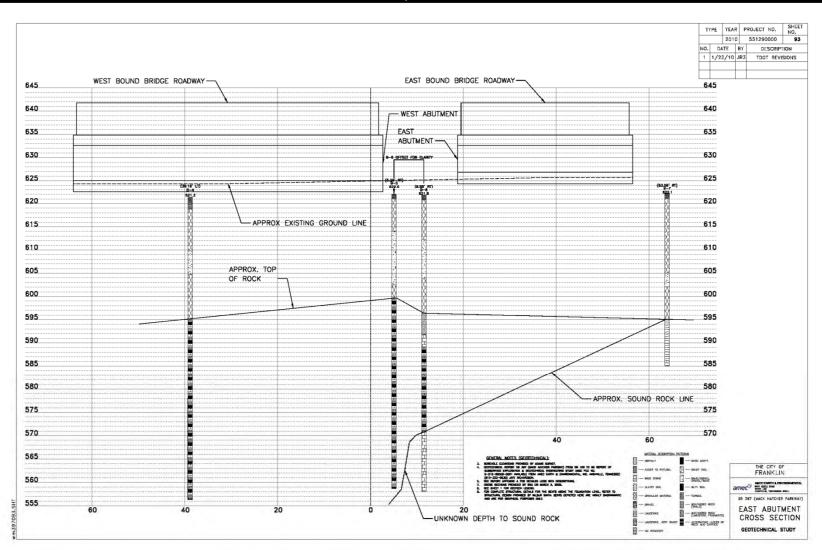




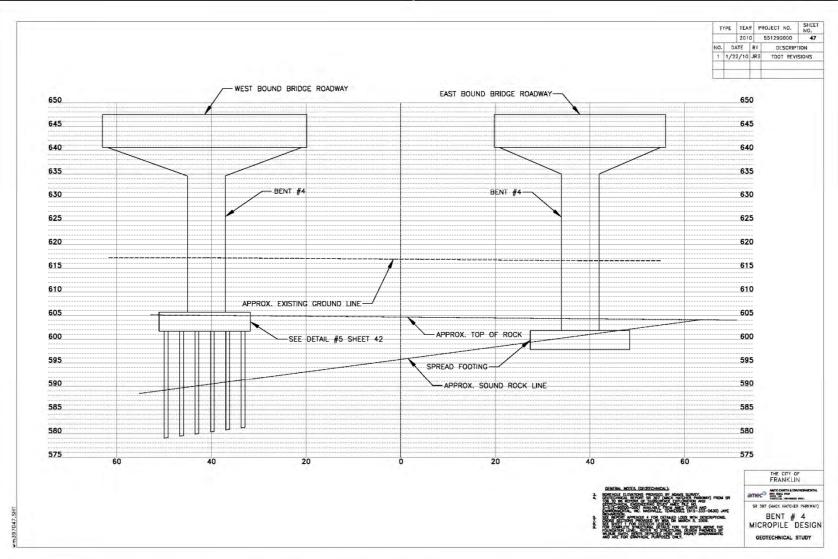


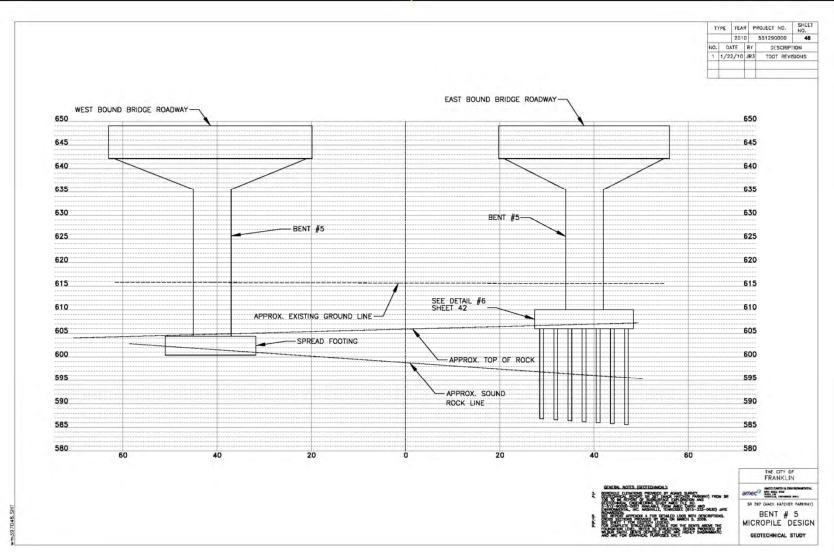


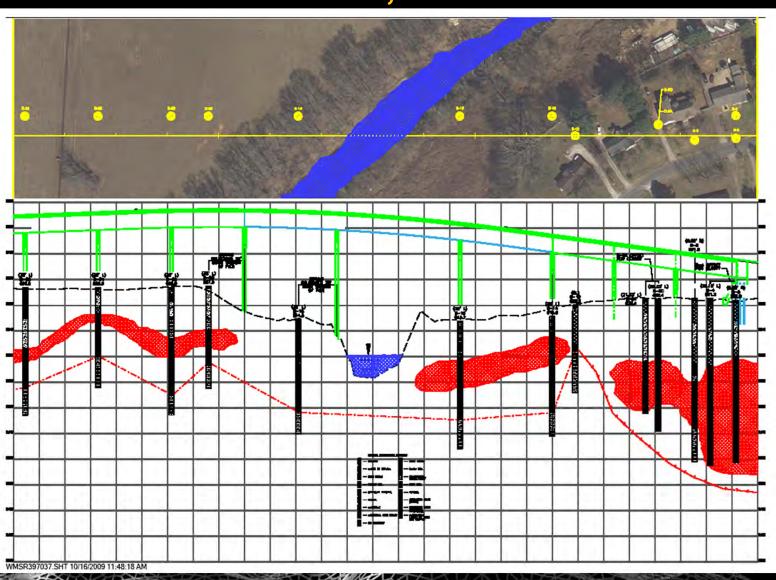




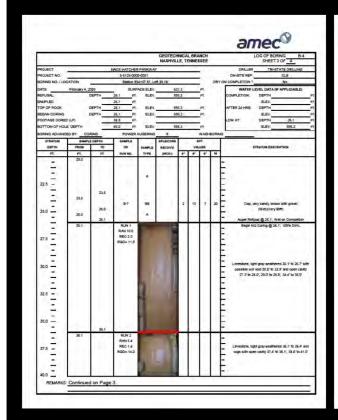


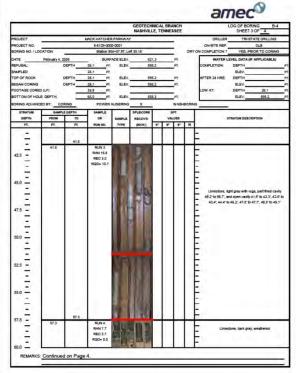


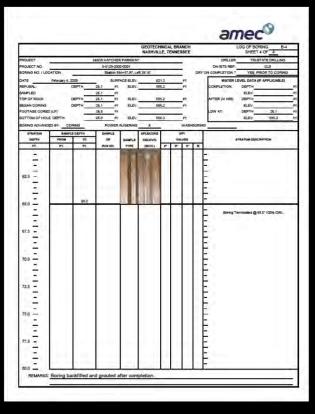


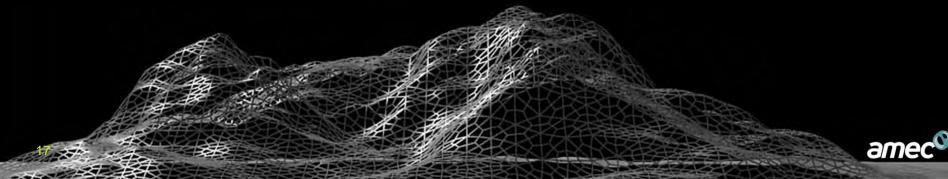


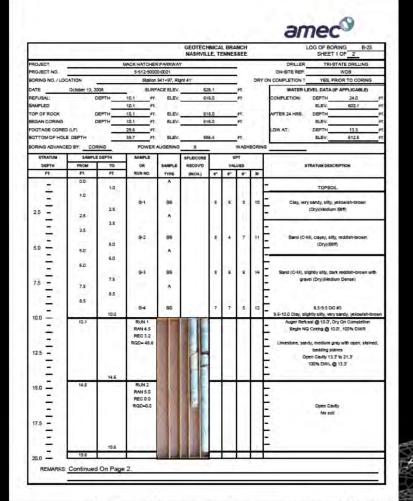












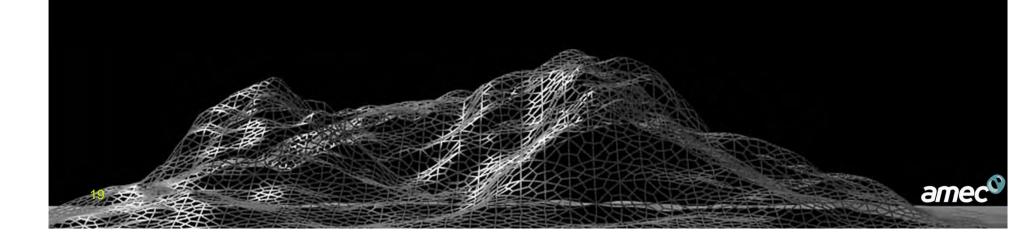


						NICAL BRANCH LE, TENNESSEE		LOG OF BORING B-25 SHEET 2 OF 2
ROJECT	MACK HATCHER PARKWAY						DRILLER TRISTATE DRILLING	
ROJECT NO.	5-512-90000-0001					ON-SITE RE		
ORING NO. / LOC	LOCATION Station 941+9			941+97, Rb	+97, Right 41'		DRY ON COMPLETION	
ATE	October 13, 20	808	SURF	ACE ELEV.	628	5.1 PT.	WATE	R LEVEL DATA (IF APPLICABLE)
FUSAL:	D.	EPTH	10.1 FT.	ELEV.	616	5,0 PT.	COMPLETION	DEPTH 24.0
WELED			10.1 FT.		_	-	1	ELEV. 602.1
OP OF ROCK	0	EPTH	10.1 FT	ELEV.	616	5.0 PT.	AFTER 24 HR	B. DEPTH
GAN CORING		EPTH	10.1 Ft.	ELEV.	616	S.D. PT.	1 10	ELEV.
OTAGE CORED			29.6 PT				LOW AT:	
TTOM OF HOLE		_	39.7 FT		586			ELEV. 512.8
RING ADVANCE			POWER.	AUGERING	X	WASH	BORING	
STRATUM	BAMPLE	DEPTH	BANFLE	1	SPLEICORE	SPT		on a course
DEPTH	FROM	10	OR	SAMPLE	RECOVE	VALUES		STRATUM DESCRIPTION
PT.	PT.	PT.	MUN NO.	TYPE	(HCH.)	4. 4. 4.	N	
	-		RUN 3	11 1			-	
-			RANS2 REC 3.3				I -	
-			RQD= 0.0	16-1	1			Continue Open Cavity to 21.3'
25 -			7.4					
_							Limestone, II	grit gray with open, stained, solution v.
-					2101		-	
_				10.6	30.00		H -	
5.0 _		24.8		м			-	
5.0 -	24.8		RUN 4		11.1			
_			RAN S.O					
			REC 5.0 RGD=96.0				-	Umestone, sandy, light gray
			NGU- 36.0	P 81 4			-	Limesione, sainty, igni gray
7.5 -				-11				
-							E E	
_							I -	
-		29.8					-	
0.0 —	29.8		RUNS	STATE OF	11 15			
	400		RAN S.D	F10 18			I E	
-			REC 5.0	ши			I F	
_			RQD= 70.0				Line	stone, sandy, light gray with open, stained, solution yags
2.5 -							<b> -</b>	statica, solution vugs
_				ми				
				en I			I E	
5.4		- 2.5		11-11	11 11 11		-	
35.D —	34.9	34.8	RUNS			+	-	
-	24.5		RAN 4.9				-	
_			REC 4.9		17 19			
= -			RGD= 87.7				Limestone	, sandy, medium to light gray with soft
7.5 -				114	THE			shale parlings
_				I II II	1 1			
					11 11 1		I -	
_								
								Terminated @ 39.7', 100% DWL



SO HOW DO WE BUILD A BRIDGE FOUNDATION ON EXTREMELY KARSTIC AND VARIABLY BEDROCK??

BY A COMBINATION OF INTEGRAL ABUTMENTS, SPREAD FOOTINGS, WHERE POSSIBLE, AND MICROPILES



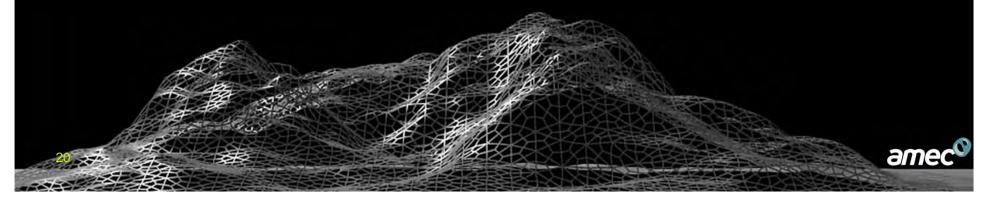
WHY?

### CAISSONS ARE DIFFICULT TO INSTALL IN IRREGULAR BEDROCK CONDITIONS

THEY ARE COSTLY \$\$

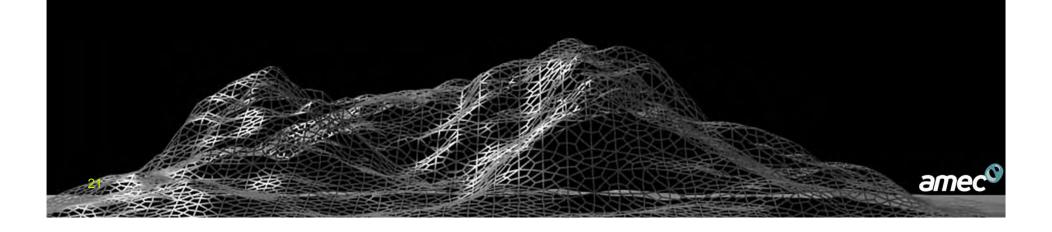
HOW MUCH ADDITIONAL CONCRETE WILL BE USED TO FILL THE OPEN CAVITIES?

HOW MUCH DOES IT COST TO USE PERMANENT CASING IN THE GROUND TO CONTROL CONCRETE PLACEMENT?

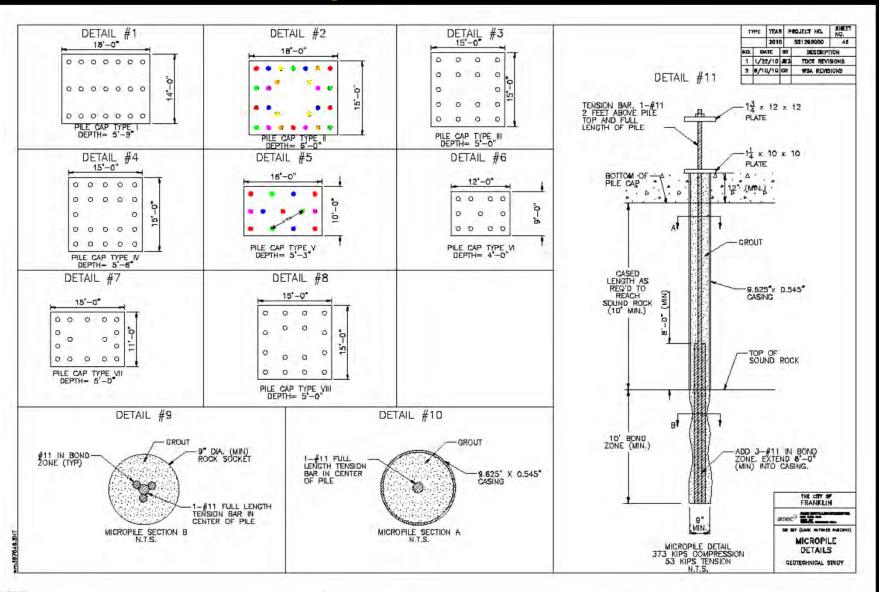


## HOW CLOSE IS THAT END BEARING H-PILE TO THE WEATHERED ROCK OR OPEN CAVITY?

WE DON'T KNOW



### Micro-piles as a SOLUTION



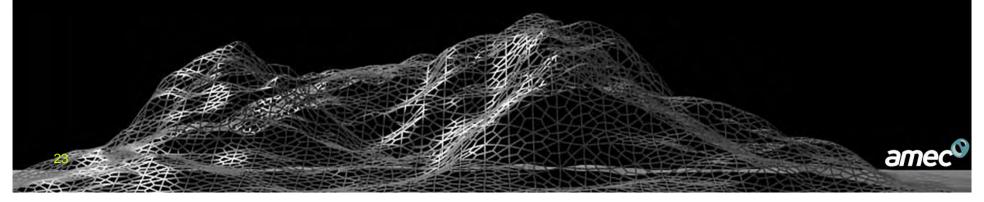
# RECENT LOAD TESTING ON INSTALLED MICRO-PILES

DESIGN LOADS WERE 300 KIPS COMPRESSION AND 180 KIPS TENSION

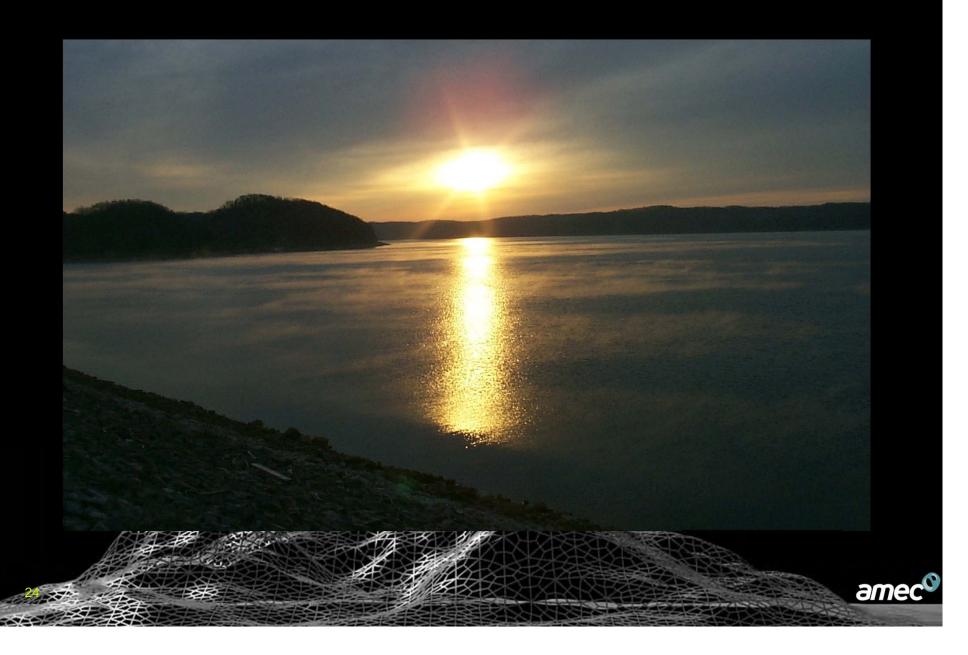
COMPRESSION LOADING EXCEEDED 200% OR 600 KIPS

TENSION LOADING FAILED AT 200% OR 360 KIPS DUE TO FOLIATION OF WEATHERED ROCK

FOLLOW-UP LOAD TESTING EXCEEDED 200% OF DESIGN FOR TENSION AND COMPRESSION



### **QUESTIONS??**



# **THANK-YOU** amec